## Solution to Problem 210D:

A pump has the following non-dimensional characteristic, $\psi(\phi)$ : where from their definitions:


$$
\begin{equation*}
\phi=\frac{Q}{A \Omega R} ; \psi=\frac{g \Delta H}{\Omega^{2} R^{2}} \tag{1}
\end{equation*}
$$

where $Q$ is the flow rate, $\Omega$ is the pump speed ( 1000 rpm ), $R$ is the impeller radius ( 15 cm ), $A$ is the pump discharge area $\left(300 \mathrm{~cm}^{2}\right), \Delta H$ is the head rise across the pump and $g$ is the acceleration due to gravity.

The pump is used to pump water from one tall tank or reservoir to another: beginning with the two

reservoirs levels at the same elevation. The cross-sectional area of the surface of both reservoirs is the same. The pipes connecting the reservoirs to the pump both have an internal diameter of 10 cm and a length of 50 m ; the appropriate friction factor, $f$, for the flow in these pipes is 0.05 .

The head loss in the pipes, $\delta H_{L}$, is given by

$$
\begin{equation*}
\Delta H_{L}=\frac{f}{2 g} \frac{L}{D}\left(\frac{Q}{A_{P}}\right)^{2} \tag{2}
\end{equation*}
$$

where $A_{P}$ is the cross-sectional area of the pipes $\left(0.03 \mathrm{~m}^{2}\right)$ and $f$ is the friction factor (0.05). Therefore teh resistance, $R$, of the piping into the pump and of the piping from the pump is given by

$$
\begin{equation*}
R=\frac{g}{Q} \Delta H_{L}=\frac{f L}{2 D} \frac{Q}{A_{P}^{2}}=2.026 \times 10^{5} Q \tag{3}
\end{equation*}
$$

The resistance of the pump, $R_{P}$, is given by $-g d H / d Q$ where

$$
\begin{equation*}
R_{P}=-g \frac{d H}{d Q} \tag{4}
\end{equation*}
$$

and since

$$
\begin{gather*}
H=\frac{\Omega^{2} R^{2}}{g}\left[0.5-8\left(\frac{Q}{A R \Omega}-0.04\right)^{2}\right]  \tag{5}\\
R_{P}=-\Omega R\left[-\frac{16}{A \Omega R}\left(\frac{Q}{A R \Omega}-0.04\right)\right]=\frac{16 Q}{A}-\frac{0.04 R \Omega}{A} \tag{6}
\end{gather*}
$$

As the head across the pump increases and the flow rate decreases the system will encounter instability when $R_{P}$ becomes equal to the pipeline resistance, $2 R_{L}$, or

$$
\begin{equation*}
R_{P}=\frac{16 Q}{A}-\frac{0.04 R \Omega}{A}=2 R=\frac{2 f L}{2 D} \frac{Q}{A_{P}^{2}} \tag{7}
\end{equation*}
$$

Solving for $Q$ this yields

$$
\begin{equation*}
Q_{i n s t a b i l i t y}=\frac{0.04 R \Omega}{A}\left[\frac{16}{A}-\frac{f L}{D A_{P}^{2}}\right]^{-1} \tag{8}
\end{equation*}
$$

Substituting the applicable parameters, this yields an instability flow coefficient, $\phi$, of 0.00168 and therefore an instability head coefficient, $\psi$, of 0.488 . This, in turn, yields an instability head rise of 12.29 m . And this would occur when the difference in the reservoir levels reached 12.27 m .

